GENERAL

G1. CODES AND STANDARDS:
2015 CHICAGO BUILDING CODE
ACI 318 AMERICAN CONCRETE INSTITUTE,
BUILDING CODE REQUIREMENTS FOR CONCRETE, LATEST EDITION
ACI 530 AMERICAN CONCRETE INSTITUTE,
BUILDING CODE REQUIREMENTS FOR MASONRY, LATEST EDITION
AISC AMERICAN INSTITUTE OF STEEL CONSTRUCTION SPECIFICATION FOR STEEL BUILDINGS, LATEST EDITION
AWS AMERICAN WELDING SOCIETY, LATEST EDITION
NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION, LATEST EDITION
ASTM AMERICAN SOCIETY OF TESTING AND MATERIALS
ANSI AMERICAN NATIONAL STANDARD INSTITUTE

G2. DESIGN LOADS:

ROOF LOAD:

DEAD LOAD:	
TRUSSES @ 16" O.C.	6 PSF
INSULATION/ROOFING	3 PSF
½" OSB	3 PSF
CEILING	5 PSF
MISC/MECH	3 PSF
TOTAL	20 PSF
SNOW LOAD:	25 PSF + DRIFT

TYPICAL FLOOR LOAD:

DEAD LOAD:	
TRUSSES @ 16" O.C.	6 PSF
FLOORING	3 PSF
½" OSB	3 PSF
CEILING	5 PSF
MISC/MECH	3 PSF
PARTITIONS	15 PSF
TOTAL	35 PSF
LIVE LOAD:	40 PSF

EXTERIOR STEEL PORCHES

DEAD LOAD:	
2" CONCRETE IN STEEL PAN	35 PSF
STEEL CHANNEL JOISTS	8 PSF
TOTAL	43 PSF
LIVE LOAD:	100 PSF

WIND LOAD:

MAIN WIND FORCE RESISTING SYSTEM	20 PSF
COMPONENTS & CLADDING:	
AT CORNERS	30 PSF
OTHER THAN AT CORNERS	25 PSF
PROJECTING ELEMENTS (BALCONIES, PARAPET, ETC)	40 PSF
NET UPLIFT ON JOISTS:	20 PSF

DEFLECTION CRITERIA:

ROOF AND FLOOR FRAMING:	
LIVE LOAD DEFLECTION	L/360
TOTAL LOAD DEFLECTION	L/240

G3. DIMENSIONS ON STRUCTURAL DRAWINGS ARE TO BE CHECKED AGAINST ARCHITECTURAL DRAWINGS AS WELL AS AGAINST FIELD CONDITIONS BY CONTRACTORS.

G4. UNLESS NOTED OTHERWISE, DETAILS, SECTIONS, AND NOTES ON THE DRAWINGS ARE INTENDED TO BE TYPICAL FOR SIMILAR CONDITIONS.

G5. IF DISCREPANCIES APPEAR ON THE CONTRACT DOCUMENTS, OR BETWEEN THE CONTRACT DOCUMENTS AND EXISTING CONDITIONS, THE CONTRACTOR SHALL REQUEST AN INTERPRETATION FROM THE ARCHITECT BEFORE BIDDING. IF THE CONTRACTOR FAILS TO MAKE SUCH REQUEST, IT IS PRESUMED THAT BOTH PROVISIONS WERE INCLUDED IN THE BID AND THE ARCHITECT SHALL DETERMINE WHICH OF THE CONFLICTING REQUIREMENTS SHALL GOVERN. THE CONTRACTOR SHALL PERFORM THE WORK AT NO ADDITIONAL COST TO THE OWNER IN ACCORDANCE WITH THE ARCHITECT'S DETERMINATION.

FOUNDATIONS

F1. ALL SOIL SUPPORTED FOOTINGS SHALL BE FOUNDED UPON UNDISTURBED, NATURAL SOIL SUBGRADE OR ON TESTED AND APPROVED FILL WITH A MINIMUM NET ALLOWABLE BEARING CAPACITY OF 3000 PSF AS INDICATED IN GEOTECHNICAL REPORT NO. 15-0157-151 BY PIONEER ENVIRONMENTAL SERVICES, LLC, DATED MAY 8, 2015 AND AS FIELD VERIFIED AND APPROVED BY THE OWNER'S SOIL TESTING LABORATORY. THE FOOTING ELEVATIONS AND SOIL BEARING CAPACITIES AS SHOWN ON THE DRAWINGS ARE ESTIMATED FROM THE SOIL BORING DATA. FINAL, EXACT ELEVATIONS AND SOIL BEARING CAPACITIES SHALL BE FIELD DETERMINED AND VERIFIED BY THE OWNER'S SOIL TESTING LABORATORY AND REVIEWED BY THE ARCHITECT/ENGINEER DURING CONSTRUCTION.

F2. THE SOIL SUBGRADE FOR ALL FOOTINGS AND SLABS SHALL BE INSPECTED AND APPROVED BY THE OWNER'S TESTING LABORATORY IMMEDIATELY PRIOR TO PLACING CONCRETE.

F3. ALL FOOTING AND SLAB SUBGRADES, INCLUDING PIT SLABS, SHALL BE COMPACTED TO 95 PERCENT OF STANDARD PROCTOR (ASTM D698) MAXIMUM DENSITY AT OPTIMUM MOISTURE CONTENT.

F4. ALL ORGANIC AND/OR OTHER UNSUITABLE MATERIALS SHALL BE REMOVED FROM SUBGRADE AND BACK FILL AREAS AND BACKFILLED WITH SELECT FILL (IDOT CA6 GRADATION), COMPACTED TO 98 PERCENT OF STANDARD PROCTOR (ASTM D698) MAXIMUM DENSITY AT OPTIMUM MOISTURE CONTENT.

F5. DO NOT UNDERMINE EXISTING CONSTRUCTION.

F6. PLACE BACKFILL SIMULTANEOUSLY ON BOTH SIDES OF FOUNDATION WALLS.

F7. NO MUD SLABS, FOOTINGS, OR SLABS SHALL BE PLACED ONTO OR AGAINST SUBGRADE CONTAINING FREE WATER, FROST, OR ICE.

F8. THE CONTRACTOR SHALL PROVIDE ALL NECESSARY MEASURES TO PREVENT ANY FROST OR ICE FROM PENETRATING ANY FOOTING OR SLAB SUBGRADE BEFORE AND AFTER PLACING OF CONCRETE UNTIL SUCH SUBGRADES ARE FULLY PROTECTED BY THE PERMANENT BUILDING STRUCTURE. SUCH PROTECTION IS ADDITIONAL SCOPE.

F9. THE CONCRETE FOR EACH ISOLATED FOOTING SHALL BE PLACED IN ONE (1) CONTINUOUS PLACEMENT.

F10. ALL PERIMETER WALL AND COLUMN FOOTINGS SHALL BEAR A MINIMUM OF 3'-6" BELOW FINISHED GRADE.

CONCRETE

C1. ALL CONCRETE WORK INCLUDING FORMING, REINFORCING, MIXING, PLACING, FINISHING AND CURING SHALL BE IN ACCORDANCE WITH THE LATEST EDITION OF THE AMERICAN CONCRETE INSTITUTE (ACI) MANUAL OF CONCRETE PRACTICE INCLUDING "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE", ACI 318 AND "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDING", ACI 301, AND ALL STANDARDS REFERENCED THEREIN.

C2. UNLESS NOTED OTHERWISE, CONCRETE SHALL BE NORMAL WEIGHT CONCRETE AND SHALL DEVELOP 3500 PSI MINIMUM COMPRESSIVE STRENGTH IN 28 DAYS. SEE TABLE BELOW FOR OTHER MIX DESIGN PARAMETERS.

C3. VERTICAL WALL CONSTRUCTION JOINTS SHALL BE FORMED WITH VERTICAL BULKHEADS AND KEYWAYS. WALL REINFORCING SHALL BE CONTINUOUS THROUGH THE JOINT OR SHALL BE DOWELED WITH AN EQUIVALENT AREA OF REINFORCEMENT.

C4. NO SLAB SHALL HAVE COLD JOINTS IN A HORIZONTAL PLANE.

C5. THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING THE LOCATION AND PLACEMENT OF INSERTS, EMBEDDED PLATES, MASONRY ANCHORS, REGLETS, SLEEVES, DUCTWORK, PADS, AND ANCHOR RODS. THE INSERTS, EMBEDDED PLATES, ETC. SHALL NOT INTERFERE WITH CONCRETE REINFORCEMENT LOCATION.

C6. NO OPENING SHALL BE MADE IN ANY STRUCTURAL MEMBER WITHOUT THE WRITTEN APPROVAL OF THE ARCHITECT.

C7. EXPOSED EXTERNAL CONCRETE CORNERS SHALL BE CHAMFERED 3/4 INCHES, UNLESS SHOWN OR NOTED OTHERWISE.

C8. SLABS ON GRADE SHALL BE PLACED IN ALTERNATE STRIPS WITH A MAXIMUM WIDTH OF 40'-0" OR AS SHOWN ON PLAN. CONTROL JOINTS SHALL BE CUT WITHIN 24 HOURS AFTER THE CONCRETE HAS SET. CONTROL JOINTS SHALL NOT EXCEED 12'-0" INTERVALS IN EACH DIRECTION, AND SHALL BE LOCATED TO CONFORM WITH BAY SPACING WHEREVER POSSIBLE (I.E. AT COLUMN CENTERLINES, HALF-BAYS, THIRD-BAYS).

C9. CONTRACTOR TO SUBMIT:
- CONCRETE MIX DESIGN FOR EACH CONCRETE MIXTURE.

C10. THE OWNER WILL ENGAGE A QUALIFIED INDEPENDENT TESTING AND INSPECTING AGENCY TO PERFORM FIELD TESTS AND INSPECTION AND PREPARE TEST REPORTS IN ACCORDANCE WITH ACI 301.

CONCRETE MIX

LOCATION	CONCRETE STRENGTH (PSI)	UNIT WEIGHT (PCF)	WATER/CEMENT RATIO (MAX)	AIR ENTRAINMENT	MIN / MAX. AGGREGATE SIZE
FOOTINGS	3,000	145	0.45	6%	¾" / 1"
FOUNDATION WALLS	4,000	145	0.45	6%	¾" / 1"
INTERIOR SLAB ON GRADE	3,500	145	0.45	0%-2%	¾" / 1"
EXTERIOR SLAB ON GRADE	4,000	145	0.4	6%	¾" / 1"

REINFORCEMENT

R1. UNLESS NOTED OTHERWISE, REINFORCEMENT SHALL CONFORM TO ASTM SPECIFICATION A615, GRADE 60.

R2. UNLESS NOTED OTHERWISE, WELDED WIRE FABRIC SHALL CONFORM TO ASTM SPECIFICATION ASTM A 185, WELDED STEEL WIRE FABRIC.

R3. CORNER BARS SHALL BE PROVIDED AT WALL CORNERS EQUAL TO THE HORIZONTAL WALL REINFORCEMENT.

R4. ALL CONCRETE FORMED SLAB OR WALL OPENINGS SHALL BE REINFORCED WITH 2 NO. 5 BARS PLACED ONE IN EACH FACE AT 45 DEGREES TO OPENING CORNERS.

R5. THE FOLLOWING CONCRETE COVER SHALL BE PROVIDED FOR REINFORCEMENT UNLESS NOTED OTHERWISE:

MINIMUM CONCRETE PROTECTION FOR REINFORCEMENT		
CONCRETE ELEMENT		MIN. COVER (IN.)
CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH		3"
CONCRETE EXPOSED TO EARTH OR WEATHER:		
#6 THROUGH #18 BARS		2"
#5 BAR, W31 OR D31 WIRE, AND SMALLER		1-1/2"
CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND		
SLABS, WALLS AND JOISTS	#14 AND #18 BARS	1-1/2"
	#11 BAR AND SMALLER	3/4"
BEAMS AND COLUMNS	PRIMARY REINFORCEMENT, TIES, STIRRUPS, SPIRALS	1-1/2"

R6. ARRANGEMENT AND DETAILS FOR REINFORCEMENT, INCLUDING BAR SUPPORTS AND SPACERS, SHALL BE IN ACCORDANCE WITH THE "A.C.I. DETAILING MANUAL (ACI SP-66)," LATEST EDITION.

R7. PROVIDE ALL ACCESSORIES NECESSARY TO SUPPORT REINFORCEMENT AT THE POSITIONS INDICATED. PLASTIC COATED ACCESSORIES SHALL BE USED IN ALL EXPOSED CONCRETE WORK.

R8. ALL EMBEDMENT LENGTHS AND LAPS SHALL BE AS REQUIRED BY ACI 318. UNLESS NOTED OTHERWISE, MINIMUM LAP SHALL BE 48 BAR DIAMETERS.

R9. CONTRACTOR TO SUBMIT SHOP DRAWINGS FOR STEEL REINFORCEMENT LAYOUT AND DETAILS, REINFORCEMENT TEST REPORTS AND CERTIFICATES.

STRUCTURAL STEEL

S1. STRUCTURAL STEEL WORK SHALL CONFORM TO THE AISC "DESIGN SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS," AND THE AISC "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES."

S2. STRUCTURAL STEEL WIDE FLANGE SHAPES SHALL CONFORM TO ASTM A992. PLATES, ANGLES, CHANNELS, AND MISCELLANEOUS MATERIAL SHALL CONFORM TO ASTM A36. HOLLOW STRUCTURAL SECTIONS SHALL CONFORM TO ASTM A500, GRADE B. STEEL PIPE SECTIONS SHALL CONFORM TO ASTM A53, GRADE B.

S3. ANCHOR RODS SHALL BE ASTM F1554 GR 55, 3/4" DIAMETER WITH EMBEDDED 12" WITH A DOUBLE NUT AT THE END OF THE ROD, UNLESS NOTED OTHERWISE.

S4. HIGH STRENGTH BOLTING SHALL BE DONE IN ACCORDANCE WITH AISC "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR BOLTS."

S5. BOLTS, NUTS, AND WASHERS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A325. BOLTS SHALL BE 3/4 INCH DIAMETER MINIMUM.

S6. WELDING SHALL BE DONE BY CERTIFIED WELDERS AND SHALL CONFORM TO AWS D1.1 "STRUCTURAL WELDING CODE-STEEL," LATEST EDITION. ALL WELDING ELECTRODES SHALL BE E70XX.

S7. THE FABRICATOR/ERECTOR SHALL SUBMIT TO THE ARCHITECT FOR REVIEW:
- ENGINEERED AND CHECKED DRAWINGS SHOWING SHOP FABRICATION DETAILS, FIELD ASSEMBLY DETAILS, AND ERECTION DIAGRAMS FOR EXTERIOR STEEL PORCHES.
- CONNECTIONS SHALL BE DESIGNED BY A LICENSED STRUCTURAL ENGINEERING IN THE STATE OF ILLINOIS. CALCULATIONS FOR EXTERIOR STEEL PORCHES PREPARED BY A LICENSED STRUCTURAL ENGINEER IN THE STATE OF ILLINOIS. DRAWINGS AND CALCULATIONS SHALL BE SUBMITTED CONCURRENTLY.

S8. FIELD CONNECTIONS, EXCEPT WHERE SHOWN TO BE WELDED, SHALL BE BOLTED.

S9. BEAMS AND JOISTS SHALL BE FABRICATED WITH THE NATURAL CAMBER UP. PROVIDE CAMBERS AS INDICATED ON THE DRAWINGS.

S10. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE CONTROL OF ALL ERECTION PROCEDURES AND SEQUENCES WITH RELATION TO TEMPERATURE DIFFERENTIALS, ESPECIALLY WITH RESPECT TO STRUCTURAL STEEL FRAMING INTO CONCRETE WALLS, BEAMS, OR COLUMNS.

S11. THERE SHALL BE NO FIELD CUTTING OF STRUCTURAL STEEL MEMBERS FOR THE WORK OF OTHER TRADES WITHOUT THE PRIOR WRITTEN APPROVAL OF THE ARCHITECT.

S12. ERECT AND MAINTAIN TEMPORARY BRACING TO INSURE THE ALIGNMENT AND STABILITY OF THE STRUCTURE DURING ERECTION UNTIL PERMANENT CONNECTIONS HAVE BEEN COMPLETED.

S13 STRUCTURAL STAINLESS STEEL BARS AND SHAPES SHALL BE _ASTM A276. ALL PLATE SHEET AND STRIPS SHALL BE ASTM 240

S14. ALL STEEL EXPOSED TO THE EXTERIOR SHALL BE GALVANIZED IN ACCORDANCE WITH A123 AND ASM A385. BOLTS, FASTENERS AND HARDWARE SHALL CONFORM TO ASTM F2329. FIELD WELDS SHALL BE GROUND SMOTH AND TOUCH UP IN ACCORDANCE WITH ASTM A780.

WOOD TRUSSES

T1. WOOD TRUSS DESIGN AND CONFIGURATION IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER. INDIVIDUAL TRUSS HEIGHT AND SPAN CONDITIONS WILL VARY FROM THE TRUSS ELEVATION DETAILS SHOWN.

T2. REFER TO THE DESIGN LOADS IN THE GENERAL NOTES FOR TRUSS DESIGN LIVE LOADS AND WIND LOADS. TRUSSES SHALL BE DESIGNED FOR WIND LOAD REACTIONS FROM PARAPET KICKERS.

T3. ROOF & FLOOR TRUSS TOP CHORD DESIGN DEAD LOAD=SEE DESIGN LOADS

T4. ROOF & FLOOR TRUSS BOTTOM CHORD DESIGN DEAD LOAD=SEE DESIGN LOADS

T5. SEE STRUCTURAL FRAMING PLANS, TRUSS ELEVATIONS, AND ARCHITECTURAL DRAWINGS FOR TRUSS BEARING LOCATIONS AND CONDITIONS.

T6. THE TRUSS MANUFACTURER SHALL COORDINATE TRUSS LAYOUT AND DESIGN WITH ALL ROOF AND FLOOR OPENINGS AND HEADERS SHOWN ON THE STRUCTURAL FRAMING PLANS AND THE ARCHITECTURAL AND MECHANICAL DRAWINGS.

T7. INTERNAL TRUSS CONNECTIONS SHALL BE DESIGNED BY THE TRUSS MANUFACTURER.

T8. TRUSS HANDLING, TEMPORARY SHORING AND PERMANENT BRIDGING AND BRACING OF TRUSSES DURING ERECTION IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER.

T9. WOOD TRUSS SHOP DRAWINGS AND CALCULATIONS, BEARING THE CERTIFICATION OF A LICENSED STRUCTURAL ENGINEER IN THE STATE OF ILLINOIS, SHALL BE SUBMITTED FOR REVIEW. THE SHOP DRAWINGS SHALL CONTAIN THE FOLLOWING INFORMATION:

DETAIL OF TRUSS WITH SIZES OF ALL MEMBERS.
SPECIES, GRADE, AND ALLOWABLE WORKING STRESSES OF LUMBER USED.
LOADING CONDITIONS USED IN TRUSS DESIGN.
CALCULATED MEMBER FORCES FOR LOADING CONDITIONS USED IN TRUSS DESIGN.
ALL TRUSS CONNECTIONS, INCLUDING CONNECTOR SIZES, CAPACITIES AND LOCATIONS.

T10. TRUSSES SHALL ALIGN WITH WALL STUDS. IF THEY DO NOT ALIGN PROVIDE ADDITIONAL STUDS/BLOCKING CONTINUOUS TO THE FOUNDATION/LOWEST LEVEL OF FRAMING. AT CONCENTRATED LOADS AND DOUBLE JOISTS PROVIDE DOUBLE STUDS U.N.O. ON PLANS AND BLOCKING CONTINUOUS TO FOUNDATION/LOWEST LEVEL OF WOOD FRAMING. ALL HEADER JAMBS AND POSTS ARE CONTINUOUS TO THE FOUNDATION/LOWEST LEVEL OF WOOD FRAMING.

WOOD NOTES

W1. ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE "TIMBER CONSTRUCTION STANDARDS" OF THE AMERICAN INSTITUTE OF TIMBER CONSTRUCTION, THE "NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION" OF THE AMERICAN FOREST AND PAPER ASSOCIATION, AND "CHAPTER 23-WOOD" OF THE INTERNATIONAL BUILDING CODE.

W2. ALL CONNECTIONS SHALL BE DESIGNED AND DETAILED BY THE CONTRACTOR FOR THE LOADS AND/OR REACTIONS SHOWN. THE CONTRACTOR SHALL SUBMIT TO THE ARCHITECT, FOR REVIEW, ENGINEERING CALCULATIONS OF ALL CONNECTIONS AND CHECKED DRAWINGS SHOWING SHOP FABRICATION DETAILS, FIELD ASSEMBLY DETAILS, AND ERECTION DIAGRAMS FOR ALL STRUCTURAL TIMBER. THE SHOP DRAWINGS AND CALCULATIONS SHALL BEAR THE CERTIFICATION OF A LICENSED STRUCTURAL ENGINEER IN THE STATE OF ILLINOIS.

W3. MATERIALS

A. SAWN LUMBER AND TIMBER

SPECIES: SOUTHERN PINE (S.P.)
GRADE: NO. 2
MODULUS OF ELASTICITY: 1,600,000 PSI
MAXIMUM IN USE MOISTURE CONTENT: 19%
MINIMUM WORKING STRESSES DRY USE CONDITION:
EXTREME FIBER IN BENDING, SINGLE MEMBER, Fb: 1250 PSI
TENSION PARALLEL TO GRAIN, Ft: 725 PSI
COMPRESSION PERPENDICULAR TO GRAIN, Fc: 565 PSI
HORIZONTAL SHEAR, Fv: 175 PSI

SPECIES: SPRUCE PINE-FIR (S.P.F.)
GRADE: NO. 1/NO. 2
MODULUS OF ELASTICITY: 1,400,000 PSI
MAXIMUM IN USE MOISTURE CONTENT: 19%
MINIMUM WORKING STRESSES DRY USE CONDITION:
EXTREME FIBER IN BENDING, SINGLE MEMBER, FB: 875 PSI
TENSION PARALLEL TO GRAIN, FC: 450PSI
COMPRESSION PERPENDICULAR TO GRAIN, FC: 425 PSI
HORIZONTAL SHEAR, FV: 135 PSI

B. PLYWOOD

GRADE: APA RATED 48/24 (ROOF), C-C EXTERIOR,
STURD-I-FLOOR 24 OC (FLOOR), C-C EXTERIOR
MODULUS OF ELASTICITY: 1,800,000 PSI
MINIMUM WORKING STRESSES DRY USE CONDITION:
EXTREME FIBER IN BENDING, Fb: 2000 PSI
TENSION IN PLANE OF PLYS, Fc: 1640 PSI
COMPRESSION IN PLANE OF PLYS, Fc: 1640 PSI
SHEAR IN PLANE PERPENDICULAR TO PLYS, Fv: 190 PSI
SHEAR IN PLANE OF PLYS, Fs: 75 PSI
BEARING PERPENDICULAR TO PLANE OF PLYS, Fc: 340 PSI

C. GLUED LAMINATED TIMBER-MICROLAM OR PARALLAM AS MANUF. BY ILEVEL WEYERHAEUSER

MINIMUM WORKING STRESSES:
BENDING:
Fb=2600 PSI
SHEAR PARALLEL TO GRAIN, Fv=285 PSI
MODULUS OF ELASTICITY, E: 2,000,000 PSI

D. TJI JOISTS SHALL BE TRUSS JOISTS BY WEYERHAEUSER. MINIMUM PROPERTIES SHALL BE AS SPECIFIED IN TRUSS JOIST TJ-4000 "SPECIFIER'S GUIDE"JULY 2018.

W4. THERE SHALL BE NO FIELD CUTTING OF STRUCTURAL TIMBER MEMBERS FOR THE WORK OF OTHER TRADES WITHOUT THE PRIOR APPROVAL OF THE ARCHITECT.

W5. NO WOOD TREATMENTS OR PRESERVATIVES SHALL BE USED WITHOUT PRIOR APPROVAL OF THE ARCHITECT.

W6. ALL MATERIAL AND FABRICATING PROCEDURES SHALL BE INSPECTED BY THE OWNER'S TESTING LABORATORY. MATERIALS SHALL BE GRADED AND MARKED IN COMPLIANCE WITH THE SPECIFICATIONS.

W7. TRUSSES SHALL ALIGN WITH WALL STUDS. IF THEY DO NOT ALIGN PROVIDE ADDITIONAL STUDS/BLOCKING CONTINUOUS TO THE FOUNDATION/LOWEST LEVEL OF FRAMING. AT CONCENTRATED LOADS AND DOUBLE JOISTS PROVIDE DOUBLE STUDS U.N.O. ON PLANS AND BLOCKING CONTINUOUS TO FOUNDATION/LOWEST LEVEL OF WOOD FRAMING. ALL HEADER JAMBS AND POSTS ARE CONTINUOUS TO THE FOUNDATION/LOWEST LEVEL OF WOOD FRAMING.

W8. ALL FASTENERS IN PRESERVATIVE TREATED WOOD SHALL BE GALVANIZED.

W9. UNLESS NOTED OTHERWISE, WOOD COMPONENTS SHALL BE FASTENED AS FOLLOWS:

FASTENING SCHEDULE - TYPICAL CONNECTION U.N.O. ON DRAWINGS		
CONNECTION	FASTENING	LOCATION
JOIST TO SILL OR GIRDER	3 - 16D COMMON	TOENAIL
BRIDGING TO JOIST	2 - 8D COMMON	TOENAIL EACH END
2" SUBFLOOR TO JOIST OR GIRDER	2 - 16D COMMON	BLIND AND FACE NAIL
SOLE PLATE TO JOIST OR BLOCKING	16D AT 16" O.C.	TYPICAL FACE NAIL
TOP PLATE TO STUD	3 - 16D COMMON	END NAIL
STUD TO SOLE PLATE	4 - 16D COMMON	TOENAIL
	3 - 16D COMMON	END NAIL
DOUBLE STUDS	(2) 16D AT 16" O.C.	FACE NAIL
DOUBLE TOP PLATES	(2) 16D AT 16" O.C.	TYPICAL FACE NAIL
	12 - 16D COMMON	LAP SPLICE
BLOCKING BETWEEN JOISTS OR RAFTERS TO TOP PLATE	16D COMMON @ 6" O.C.	TOENAIL
RIM JOIST TO TOP PLATE	16D AT 6" O.C.	TOENAIL
TOP PLATES, LAP, AND INTERSECTIONS	4 - 16D COMMON	FACE NAIL
CONTINUOUS HEADER, TWO PIECES	16D - ROWS SPACED 2" COL'S SPACED 16"	FACE NAIL
CEILING JOISTS TO PLATE	3 - 16D COMMON	TOENAIL
CONTINUOUS HEADER TO STUD	4 - 16D COMMON	TOENAIL
CEILING JOISTS TO PARALLEL RAFTERS	4 - 16D COMMON, MIN.	FACE NAIL
RAFTER TO PLATE	3 - 16D COMMON	TOENAIL
BUILT-UP CORNER STUDS	16D COMMON	16" O.C.
BUILT-UP GIRDER AND BEAMS	20D - ROWS SPACED 2" COL'S SPACED 16"	FACE NAIL STAGGERED ON OPPOSITE SIDES
	4 - 20D COMMON	FACE NAIL AT ENDS AND AT EACH SPLICE
COLLAR TIE TO RAFTER	3 - 16D COMMON	FACE NAIL
JACK RAFTER TO HIP	3 - 16D COMMON	TOENAIL
	2 - 16D COMMON	FACE NAIL
ROOF RAFTER TO 2-BY RIDGE BEAM	2 - 16D COMMON	TOENAIL
	2 - 16D COMMON	FACE NAIL
JOIST TO BAND JOIST	3 - 16D COMMON	FACE NAIL
LEDGER STRIP	3 - 16D COMMON	FACE NAIL
	4 - 3" x 0.131" NAILS	FACE NAIL
	4 - 3" 14 GAGE STAPLES	FACE NAIL
WOOD STRUCTURAL PANELS AND PARTICLEBOARD	1/2" AND LESS, 8D	
SUBFLOOR, ROOF, AND WALL SHEATHING (TO FRAMING)	19/32" TO 3/4", 8D;	
SINGLE FLOOR, COMBINATION SUBFLOOR-UNDERLAYMENT TO FRAMING	1 1/8" TO 1 1/4", 10D;8D	
	3/4" AND LESS, 8D	
	7/8" TO 1", 8D	
PANEL SIDING (TO FRAMING)	1/2" OR LESS, 8D	
	5/8", 8D	

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245 Sidney Court
Villa Park, Illinois 60181
630.833.8600 Fax 630.833.9595

CONSULTANTS

STATEMENT OF COMPLIANCE
I HAVE PREPARED, OR CAUSED TO BE PREPARED UNDER MY DIRECT SUPERVISION, THE ATTACHED PLANS AND SPECIFICATIONS AND STATE THAT, TO THE BEST OF MY KNOWLEDGE AND BELIEF AND TO THE EXTENT OF MY CONTRACTUAL OBLIGATION, THEY ARE IN COMPLIANCE WITH ALL THE APPLICABLE CODES, INCLUDING THE ENVIRONMENTAL BARRIERS ACT (410 ILCS) AND THE 2010 AMERICAN WITH DISABILITIES ACT

HAMMOND, INDIANA

progress set - 10-18-17
not for construction

ILLINOIS DESIGN FIRM NO. 184-005411

ISSUE	DATE
FOR REVIEW	08/06/10
PROGRESS	12/31/18
PROGRESS	01/09/19

DRAWN BY:	
APPROVED:	
PROJECT NO.	
DATE	01/16/19

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ST. MARGARET MARY PARISH SPIRITUAL CENTER

1445 Hoffman Street
Hammond, Indiana

SHEET DESCRIPTION
GENERAL NOTES

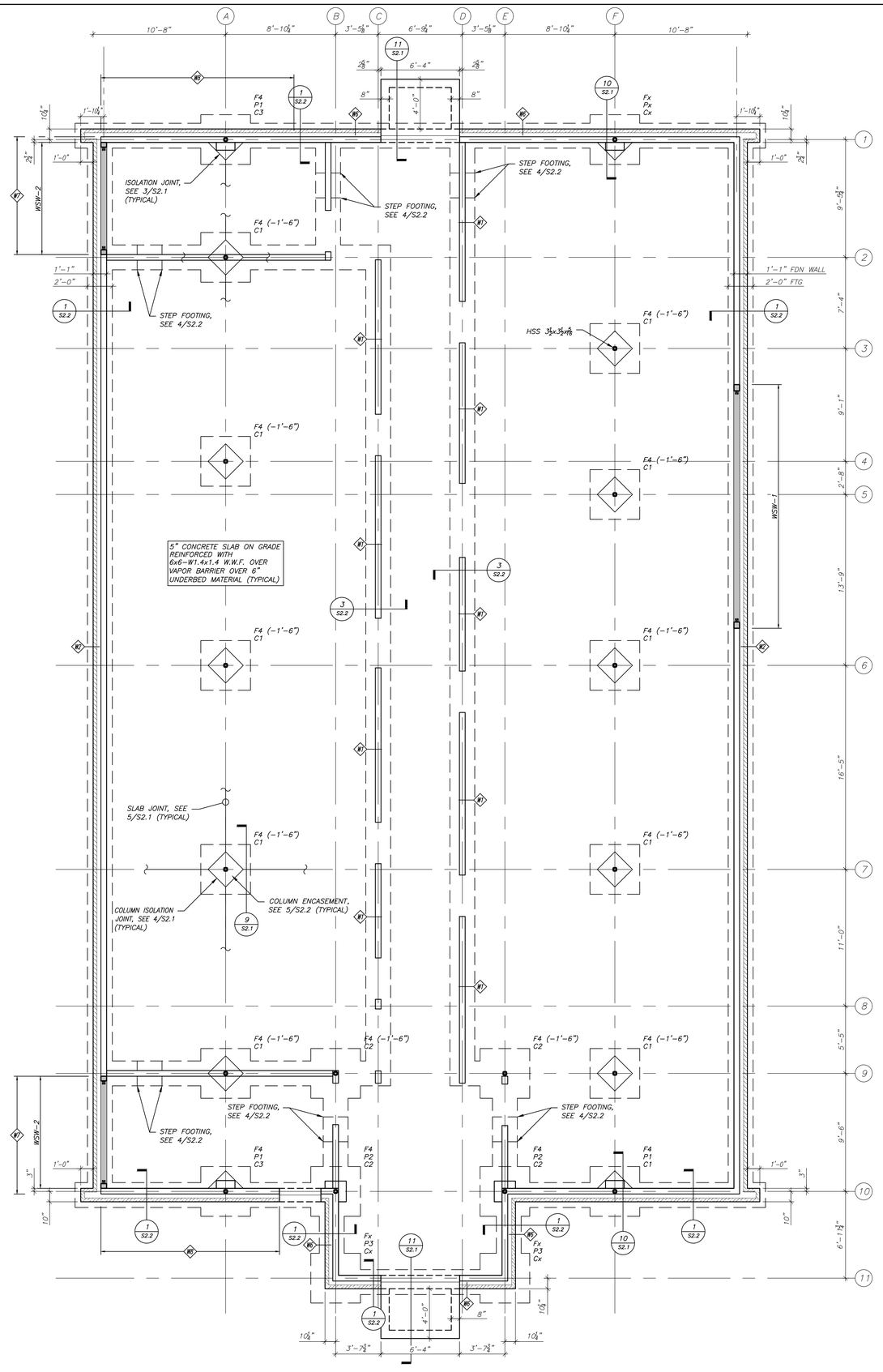
SHEET NUMBER
S0.1
SHEET X of X

NOTES:

1. Fx (x'-x") INDICATES FOOTING MARK AND ELEVATION. SEE SHEET S2.1 FOR FOOTING SCHEDULES AND DETAILS.
2. Px (x'-x") INDICATES PIER MARK AND ELEVATION. SEE SHEET S2.1 FOR PIER DETAILS.
3. Cx INDICATES COLUMN MARK. SEE SHEET S3.1 FOR COLUMN SCHEDULE AND BASE PLATE DETAILS.
4. T/FOOTING = -3'-0" U.N.O.
5. T/SLAB ON GRADE = 0'-0" U.N.O.
6. T/PIER = -1'-0" U.N.O.
7. SEE S0.1 FOR GENERAL NOTES.
8. SEE SHEETS S2.1 AND S2.2 FOR TYPICAL FOUNDATION DETAILS.
9. SEE SHEET S3.1 FOR WALL SCHEDULE.
10. THICKENED SLAB ON GRADE PER DETAIL 3/S2.2.
11. (W1) = WOOD STUD WALL, SEE S3.1 FOR WALL SCHEDULE.

= 6" WOOD STUD SHEAR WALL, REFER TO x/Sx.x

= VENEER, SEE ARCH.



1 S1.1 FOUNDATION & 1ST FLOOR FRAMING PLAN

SCALE: 1/4" = 1'-0"

0 2' 4' 8'

SCALE: 1/4" = 1'-0"

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CONSULTANTS

STATEMENT OF COMPLIANCE

I HAVE PREPARED, OR CAUSED TO BE PREPARED UNDER MY DIRECT SUPERVISION, THE ATTACHED PLANS AND SPECIFICATIONS AND STATE THAT, TO THE BEST OF MY KNOWLEDGE AND BELIEF AND TO THE EXTENT OF MY CONTRACTUAL OBLIGATION, THEY ARE IN COMPLIANCE WITH ALL THE APPLICABLE CODES, INCLUDING THE ENVIRONMENTAL BARRIERS ACT (410 ILCS) AND THE 2010 AMERICAN WITH DISABILITIES ACT

HAMMOND, INDIANA

progress set - 10-18-17
not for construction

ILLINOIS DESIGN FIRM NO. 184-005411

ISSUE	DATE
FOR REVIEW	08/06/10
PROGRESS	12/31/18
PROGRESS	01/09/19

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PROJECT NO.	
DATE	01/16/19

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ST. MARGARET MARY PARISH SPIRITUAL CENTER

1445 Hoffman Street
Hammond, Indiana

SHEET DESCRIPTION

FOUNDATION/ FIRST FLOOR FRAMING PLAN

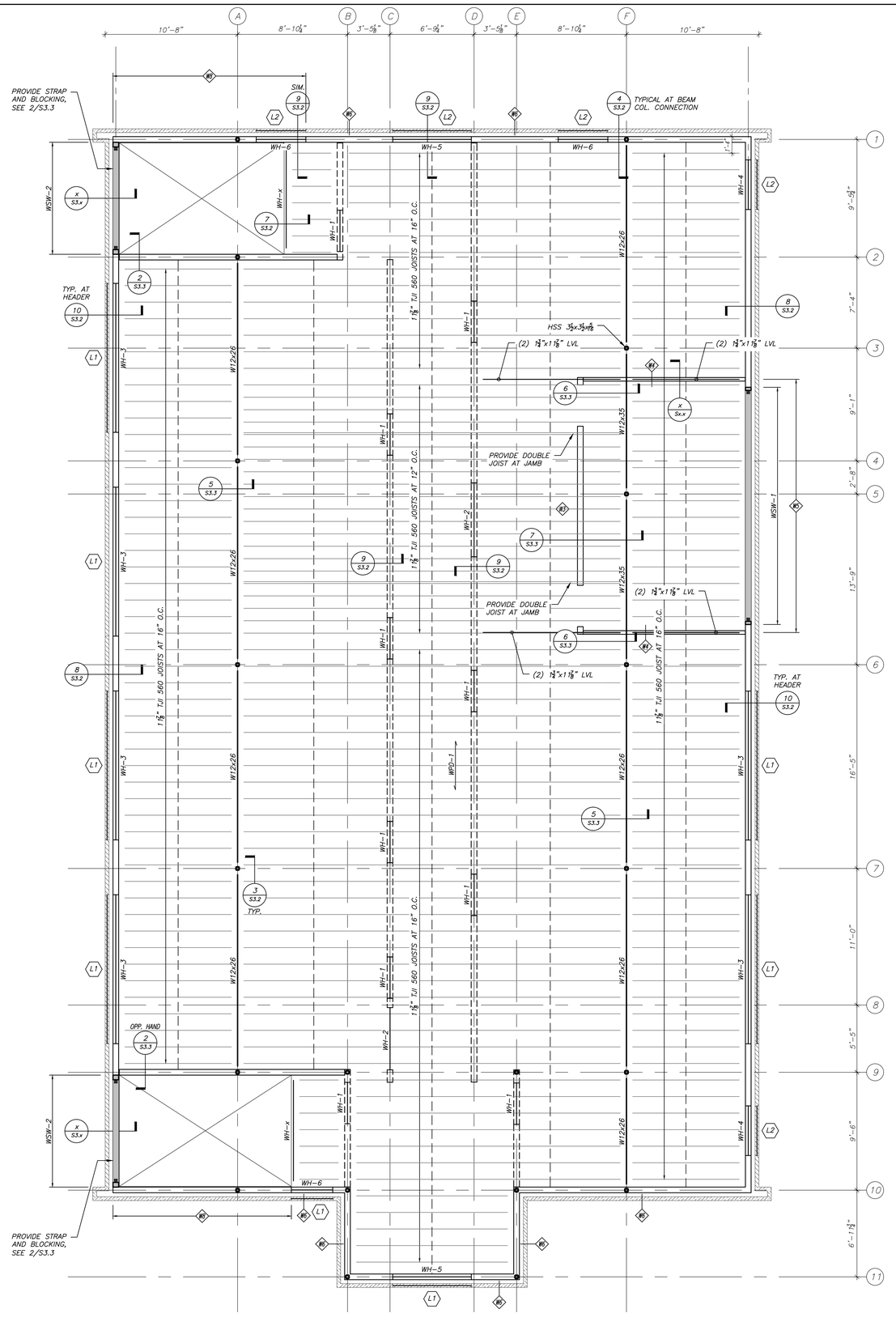
SHEET NUMBER

S1.1

SHEET X of X

NOTES:

1. T/FLOOR SHEATHING = 11'-1"
2. T/STEEL ELEV. = **x'-x"**
3. WPD-x INDICATES SPAN DIRECTION OF WOOD PANEL DIAPHRAGM. PROVIDE 23/32", APA RATED, 24" O.C., SEE X/Sx.x DIAPHRAGM FASTENING SCHEDULE. BOTTOM LAYER OF SHEATHING SHALL BE STRUCTURAL DIAGHRAM AND TOP LAYER OF ¾" SHEATHING TO ACT AS UNDERLAYMENT. SEE ARCHITECTURAL SECTIONS.
4. SEE SHEETS S3.x AND S3.x FOR TYPICAL SUPERSTRUCTURE. DETAILS. SEE SHEET S3.x FOR LATERAL BRACING ELEVATIONS AND DETAILS.
5. Lx INDICATES LINTEL MARK. SEE SHEET S3.x FOR LINTEL DETAILS.
6. WH-x INDICATES WOOD HEADER. SEE **xxxxxxxxxx.**



1 S1.2 SECOND FLOOR FRAMING PLAN

SCALE: 1/4" = 1'-0"

0 2' 4' 8'

SCALE: 1/4" = 1'-0"

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SHEET DESCRIPTION

SECOND FLOOR FRAMING PLAN

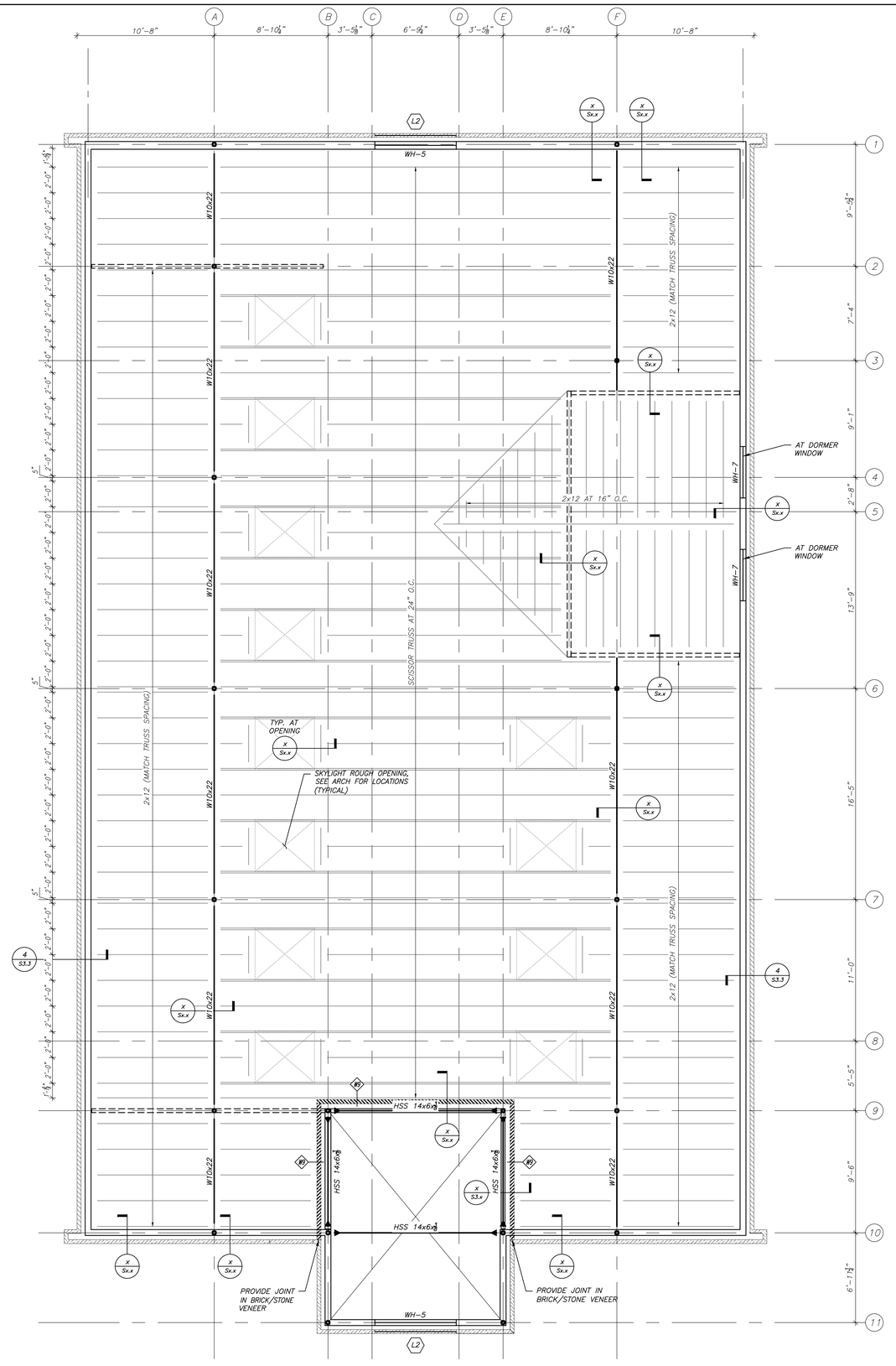
SHEET NUMBER

S1.2

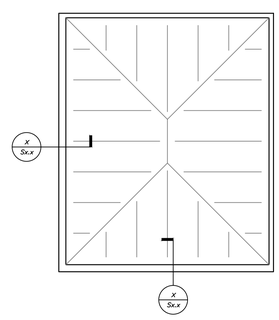
SHEET X of X

NOTES:
1. T/SLAB EL. x'-x"
2. T/STEEL EL: x'-x"
3. ⌒ MSx ⌒ INDICATES SPAN DIRECTION OF COMPOSITE METAL DECK. SEE SHEET Sx.x FOR DECK SCHEDULE AND TYP. DETAILS
4. ALL STEEL SHALL BE EVENLY SPACED IN BAYS, U.N.O.
5. (X) INDICATES NUMBER OF ¾" DIA. x 4" LONG HEADED SHEAR STUDS EVENLY SPACED
6. <c = x"> INDICATES BEAM CAMBER.
7. SEE SHEETS Sx.x AND Sx.x FOR TYPICAL SUPERSTRUCTURE DETAILS. SEE SHEET S3.x FOR LATERAL BRACING ELEVATIONS AND DETAILS.
8. Lx INDICATES LINTEL MARK. SEE SHEET Sx.x FOR LINTEL DETAILS.

FINISH THIS NOTE:
1. SCISSOR TRUSS PLAN NOTE (LIMIT HORIZONTAL DEFLECTION)



1 S1.3 ROOF FRAMING PLAN
SCALE: 1/4" = 1'-0"



2 S1.3 TOWER ROOF PLAN
SCALE: 1/4" = 1'-0"

0 2' 4' 8'
SCALE: 1/4" = 1'-0"

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SHEET DESCRIPTION
ROOF FRAMING PLAN

SHEET NUMBER
S1.3
SHEET X of X

0 1' 2' 4'

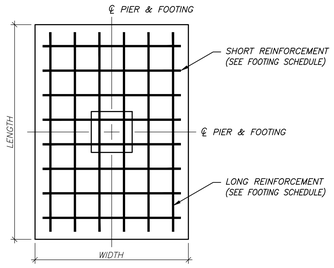
SCALE: 1/2" = 1'-0"

FOOTING SCHEDULE

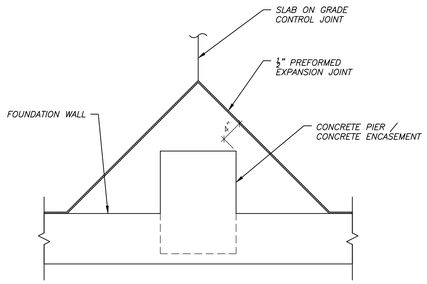
Fb = SEE STRUCT. NOTES
f'c = 3,000 PSI
fy = 60,000 PSI

FOOTING MARK	SIZE			BOTTOM REINFORCEMENT		TOP REINFORCEMENT		REMARKS
	WIDTH	LENGTH	DEPTH	LONG.	SHORT	LONG.	SHORT	
F4	4'-0"	4'-0"	1'-2"	(5) #5	(5) #5	–	–	SEE x/Sx.x
F5	5'-0"	5'-0"	1'-2"	(6) #6	(6) #6	–	–	SEE x/Sx.x
F6	6'-0"	6'-0"	1'-4"	(7) #6	(7) #6	–	–	SEE x/Sx.x
F7	7'-0"	7'-0"	1'-6"	(8) #7	(8) #7	–	–	SEE x/Sx.x
F8	8'-0"	8'-0"	2'-0"	(9) #8	(9) #8	–	–	SEE x/Sx.x
F9	9'-0"	9'-0"	2'-0"	(10) #8	(10) #8	–	–	SEE x/Sx.x
F10	10'-0"	10'-0"	2'-4"	(11) #9	(11) #9	–	–	SEE x/Sx.x
F11	11'-0"	11'-0"	2'-6"	(12) #9	(12) #9	–	–	SEE x/Sx.x
F12	12'-0"	12'-0"	3'-0"	(13) #10	(13) #10	–	–	SEE x/Sx.x

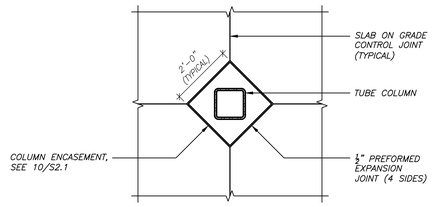
1 / S2.1 FOOTING SCHEDULE
SCALE: NOT APPLICABLE



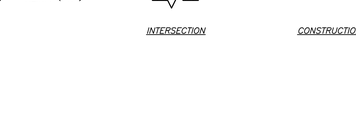
2 / S2.1 TYP. RECTANGULAR FOOTING PLAN
SCALE: NOT TO SCALE



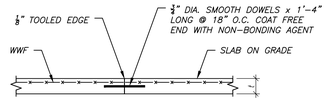
3 / S2.1 TYPICAL EXTERIOR COLUMN ISOLATION JOINT DETAIL



4 / S2.1 TYPICAL INTERIOR COLUMN ISOLATION JOINT DETAIL



CONTROL JOINT



CONSTRUCTION JOINT

5 / S2.1 TYPICAL SLAB ON GRADE JOINT DETAILS

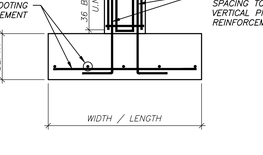
CONCRETE PIER SCHEDULE

PIER MARK	TYPE	SIZE A x B	REINFORCEMENT VERTICAL	REINFORCEMENT TIES	REMARKS
P1	1	18" x 22½"	(6) #8	#4 AT 12"	ADD (2) TIES AT TOP
P2	2	X" x X"	(x) #X	#x AT x"	–
P3	3	X" x X"	(x) #X	#x AT x"	–

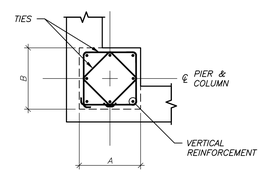
NOTES:

1. SEE FOUNDATION PLANS AND DETAILS FOR TOP OF PIER ELEVATIONS.
2. SEE FOUNDATION WALL SECTIONS FOR WALL REINFORCEMENT.
3. DASHED LINE ON PIER DETAILS INDICATES LIMITS OF DEPRESSED AREA FOR COLUMN BASE. FILL ALL POCKETS WITH CONCRETE AFTER THE STEEL FRAME IS ERECTED AND PLUMB.

6 / S2.1 CONCRETE PIER SCHEDULE

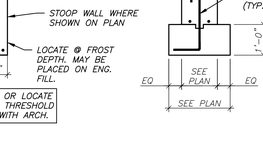


PIER TYPE 1

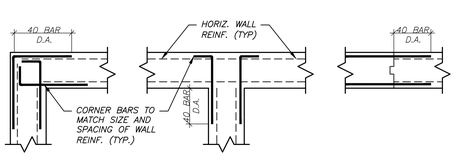


PIER TYPE 3

7 / S2.1 PIER TYPE 1 THRU 3
SCALE:

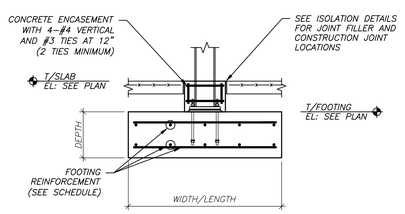


PIER TYPE 2



CORNER INTERSECTION CONSTRUCTION JOINT

8 / S2.1 TYP. WALL REINFORCING DETAIL
SCALE: NOT TO SCALE



9 / S2.1 TYPICAL PIER DETAIL AT DEPRESSED COLUMN BASE

10 / S2.1 TYPICAL PIER & FOOTING DETAIL

11 / S2.1 TYPICAL FOUNDATION WALL SECTION AT DOOR

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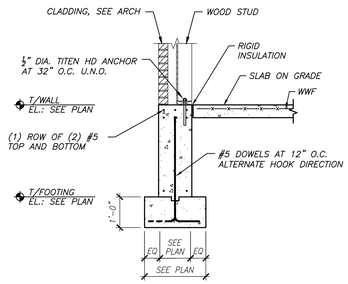
SHEET DESCRIPTION
FOUNDATION DETAILS

SHEET NUMBER
S2.1
SHEET X of X

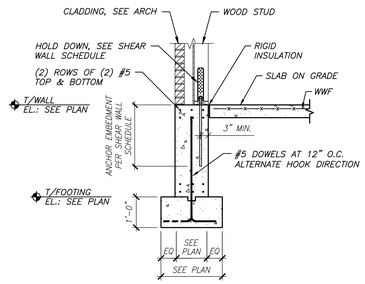
0 1' 2'

SCALE: 1" = 1'-0"



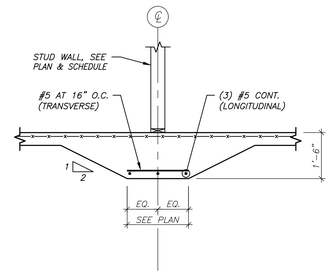
1 S2.2 TYP. FOUNDATION WALL SECTION

SCALE: NOT TO SCALE



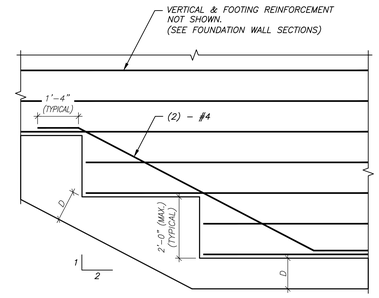
2 S2.2 FOUNDATION WALL SECTION AT HOLD-DOWN

SCALE: 1/2" = 1'-0"



3 S2.2 TITLE

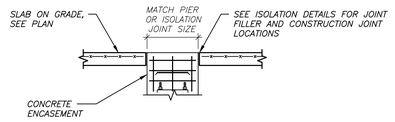
SCALE: 1/2" = 1'-0"



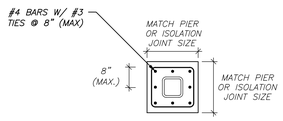
D = FOOTING DEPTH (SEE FDN WALL SECTIONS)

4 S2.2 TYP. STEPPED FOOTING DETAIL

SCALE: 1/2" = 1'-0"



SECTION



PLAN VIEW

5 S2.2 TYP. COLUMN ENCASEMENT DETAIL

SCALE: 1/2" = 1'-0"

0 1' 2' 4'

SCALE: 1/2" = 1'-0"

0 6" 1' 2'

SCALE: 1-1/2" = 1'-0"

0 1' 2' 3'

SCALE: 3/4" = 1'-0"

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SHEET DESCRIPTION

FOUNDATION DETAILS

SHEET NUMBER

S2.2

SHEET X of X

SHEAR WALL SCHEDULE

MARK	LEVEL	WALL STUD SIZE	END POST	SIMPSON HOLD DOWN	SHEATHING	PANEL EDGE FASTENER SIZE AND SPACING	SILL PLATE FASTENERS	TOP PLATE BOUNDARY FASTENERS	BASE HOLD DOWN ANCHOR/EMBEDMENT
WSW-1	2	SEE PLAN & SCHEDULE	(2) 2x6 LVL	HDU5-SDS2.5	15/32" OSB	10d NAILS AT 6" O.C.	(2) 16d NAILS AT 6" O.C.	(2) 16d NAILS AT 6" O.C.	5/8" THREADED ROD
	1	SEE PLAN & SCHEDULE	(4) 2x6 SPF #1/2	HDU11-SDS2.5	15/32" OSB	10d NAILS AT 4" O.C.	½" DIA. TITEN HD AT 12" O.C.	(2) 16d NAILS AT 4" O.C.	1" HAS ROD WITH 16" EMBEDMENT
WSW-2	1	SEE PLAN & SCHEDULE	(3) 2x6 SPF #1/2	HDU8-SDS2.5	15/32" OSB	10d NAILS AT 6" O.C.	½" DIA. TITEN HD AT 12" O.C.	(2) 16d NAILS AT 4" O.C.	5/8" (0.625") HAS ROD WITH 16" EMBEDMENT

NOTES:
1. MINIMUM NAIL PENETRATION = 1½"
2. SEE WALL SCHEDULE FOR TOP & BOTTOM.
3. BOTTOM PLATES SHALL BE LVL FOR INTERIOR SHEAR WALLS.
4. BASE ANCHOR EMBEDMENT IS EMBEDMENT INTO FOUNDATION WALL.
5. EPOXY ANCHORS REQUIRE DEEP EMBEDMENT PROCEDURES AS RECOMMENDED BY MANUFACTURER. EPOXY SHALL BE HILTI HIT-RE 500 OR EQUIVALENT.

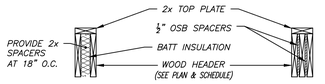
WOOD HEADER SCHEDULE

MARK	SIZE	QUANTITY	TYPE	BEARING STUDS	KING STUDS	REMARKS
H1	2x10	2	SAWN	1	2	
H2	2x8	2	SAWN	1	1	
H3	2x12	3	SAWN	2	1	
H4	1.75 x 9.25	2	LVL	2	2	
H5	2x12	2	SAWN	1	1	
H6	1.75 x 9.25	3	LVL	3	1	
H7	1.75 x 11.25	2	LVL	1	2	
H8	2x8	2	SAWN	1	4	FULL HT LSL STUDS
H9	1.75 x 9.25	3	LVL	1	1	
H10	1.75 x 14	3	LVL	3	1	

HEADERS SHALL BE SPF #1/#2, OR BETTER FOR SAWN LUMBER, TRUSS JOIST MICROLLAM 1.9E FOR LVL

ALL STUDS SHALL BE SPRUCE-PINE FIR #1/#2 OR BETTER LVL'S LESS THAN OR EQUAL TO 12" DEEP:
(2) ROWS OF 16d AT 12'-0" O.C. FOR HEADER ATTACHMENT
LVL'S GREATER THAN 12" DEEP:
(3) ROWS OF 16d AT 12" O.C. FOR HEADER ATTACHMENT

PROVIDE 2x KING STUD CONTINUOUS FROM SILL PLATE TO TOP PLATE ADJACENT TO BEARING STUDS (TYPICAL). REFERENCE SCHEDULE FOR QUANTITY. NAILING PER SCHEDULE.



2 PLY HEADER 3 PLY HEADER

HEADER NOTE
TOP & BOTTOM PLATES & INSULATION ONLY APPLIES TO HEADERS IN EXTERIOR WALLS OR NOT DIRECTLY BELOW TOP PLATES OF THE TOP OF THE WALL

WOOD POST SCHEDULE

MARK	LEVEL	POST SIZE	NOTES
WP1	–	–	CONTINUATION OF TRIMMER STUDS BELOW OPENINGS ABOVE. CONTINUE POST TO FOUNDATION
WP2	–	–	CONTINUATION OF SHEAR WALL END POST ABOVE. PROVIDE SIMPSON HDU11-SDS2.5 HOLDOWN AT EACH FLOOR.
WP3	–	–	

SCALE: 3/4" = 1'-0"

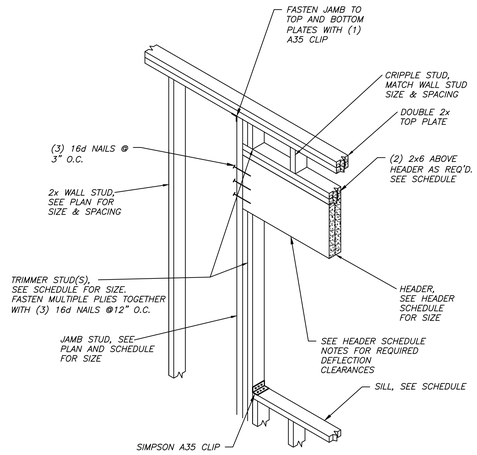
1 S3.1 SHEAR WALL SCHEDULE
SCALE: NOT APPLICABLE

WALL SCHEDULE

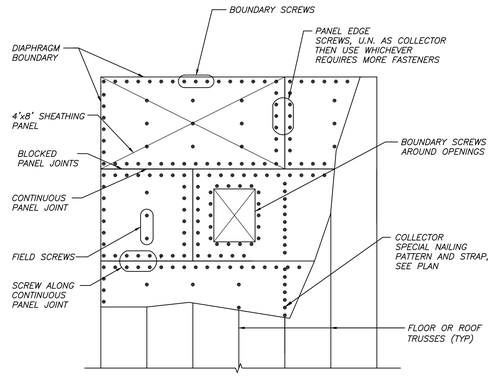
MARK	LEVEL	WALL TYPE	STUD SIZE AND SPACING	SPECIES/GRADE	TOP PLATE SIZE AND SPECIES	BOTTOM PLATE SIZE AND SPECIES	REMARKS
W1	1	LOAD BEARING INTERIOR	2x6 AT 16" O.C.	SPF #1/2	(2) 2x6 SPF #1/2	(1) 2x6 SPF #1/2	
W2	1	LOAD BEARING EXTERIOR	2x6 AT 16" O.C.	SPF #1/2	(2) 2x6 SPF #1/2	(1) 2x6 SPF #1/2	
W3	2	LOAD BEARING INTERIOR	2x6 AT 16" O.C.	SPF #1/2	(2) 2x6 SPF #1/2	(2) 2x6 SPF #1/2	
W4	2	LOAD BEARING EXTERIOR	2x4 AT 16" O.C.	SPF #1/2	(2) 2x4 SPF #1/2	(1) 2x4 SPF #1/2	
W5	2	NON-LOAD BEARING EXTERIOR	2x6 AT 16" O.C.	LVL	(2) 2x6 SPF #1/2	(1) 2x6 SPF #1/2	
W6	2	NON-LOAD BEARING EXTERIOR	2x8 AT 16" O.C.	LVL	(2) 2x8 SPF #1/2	(1) 2x8 SPF #1/2	
	1	NON-LOAD BEARING EXTERIOR	2x6 AT 16" O.C.	SPF #1/2	(2) 2x4 SPF #1/2	(1) 2x6 SPF #1/2	
W7	1 – ROOF	LOAD BEARING EXTERIOR	(2) 2x6 AT 16" O.C.	SPF #1/2	(2) 2x6 SPF #1/2	(1) 2x6 SPF #1/2	
W8	1 – ROOF	NON-LOAD BEARING EXTERIOR	(2) 2x8 AT 16" O.C.	LVL	(2) 2x8 SPF #1/2	(1) 2x8 SPF #1/2	
W9	ROOF – HIGH ROOF	LOAD BEARING EXTERIOR	2x6 AT 16" O.C.	SPF #1/2	(2) 2x6 SPF #1/2	(1) 2x6 SPF #1/2	

NOTES:
1. ALIGN STUDS W/ JOISTS
2. SPF = SPRUCE PINE FIR
3. ALL PLATES ARE SPF #1/2 WIDTH TO MATCH STUD WIDTH U.N.O.
4. REFER TO **6/S3.1** FOR ADDITIONAL NOTES AND DETAILS.

2 S3.1 WOOD HEADER SCHEDULE
SCALE: NOT APPLICABLE



3 S3.1 WOOD POST SCHEDULE
SCALE: NOT APPLICABLE



4 S3.1 WALL SCHEDULE
SCALE: NOT APPLICABLE

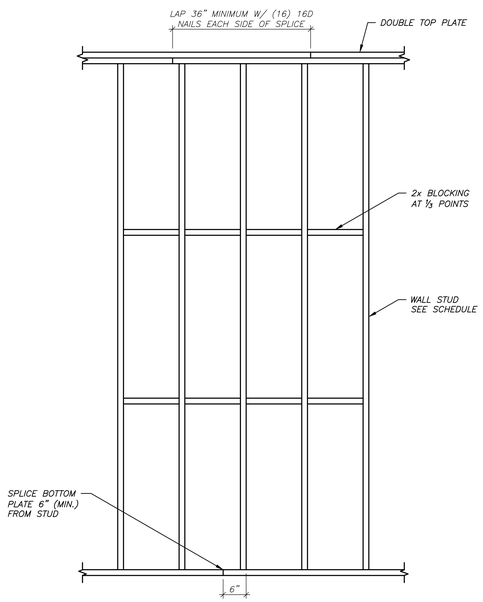
5 S3.1 TYP. BEARING WALL HEADER DETAIL
SCALE: NOT APPLICABLE

6 S3.1 DIAPHRAGM FASTENING DETAIL
SCALE: NOT TO SCALE

DIAPHRAGM FASTENING SCHEDULE

SCREW LOCATION	SCREW SIZE	SCREW SPACING
BOUNDARY SCREWS	#10	4"
PANEL EDGE SCREWS	#10	4"
FIELD SCREWS	#10	12"
SCREWS ALONG PANEL JOINT	#10	4"
–	–	–

NOTES:
1. USE APA RATED SHEATHING.
2. USE SCREW SCHEDULE FOR ALL FLOORS & ROOF DIAPHRAGM U.N.O.
3. MINIMUM FASTENER PENETRATION IN FRAMING IS 1½".



7 S3.1 DIAPHRAGM FASTENING SCHEDULE
SCALE: NOT APPLICABLE

7 S3.1 TYP. BEARING WALL ELEVATION
SCALE: NOT TO SCALE

HEADER SCHEDULE

MARK	OPENING WIDTH	LEVEL	HEADER SIZE	HEADER SPECIES/GRADE	SILL SIZE	SILL SPECIES/GRADE	TRIMMER STUD SIZE	TRIMMER STUD SPECIES/GRADE	KING STUD SIZE	KING STUD SPECIES/GRADE	NOTES/ REMARKS
WH-1	3'-6" (MAX.)										
		1	(2) 2x12	SPF #1/2			(2) 2x6	SPF #1/2	2x6	SPF #1/2	
WH-2	6'-0" (MAX.)										
		1	(2) 1¾"x11⅞"	LVL			(3) 2x6	SPF #1/2	2x6	SPF #1/2	
WH-3	12'-0" (MAX.)										
		1	(3) 1¾"x11⅞"	LVL	(3) 2x6 LVL		(2) 2x6	SPF #1/2	(3) 2x6	SPF #1/2	
WH-4	4'-6" (MAX.)										
		1	(2) 2x12	SPF #1/2	(1) 2x6	SPF #1/2	(2) 2x6	SPF #1/2	(2) 2x6	SPF #1/2	
WH-5	6'-6" (MAX.)	2	(3) 1¾"x11⅞"	LVL	(2) 2x6	SPF #1/2	(2) 2x8	LVL	(6) 2x8	LVL	
		1	(3) 2x12	SPF #1/2	(2) 2x6	SPF #1/2	(2) 2x6	SPF #1/2	(3) 2x6	SPF #1/2	
WH-6	6'-8" (MAX.)										
		1	(3) 2x12	SPF #1/2	(2) 2x6	SPF #1/2	(2) 2x6	SPF #1/2	(3) 2x6	SPF #1/2	
WH-7	6'-6" (MAX.)	2	(3) 2x12	SPF #1/2	(2) 2x6	SPF #1/2	(2) 2x6	SPF #1/2	(4) 2x6	SPF #1/2	

NOTES:
1. ALL HEADERS AND MEMBERS MUST SPAN ENTIRE LENGTH OF OPENING WITH NO SPLICES.
2. ALL HEADER MATERIAL SHALL BE UNPUNCHED
3. REFER TO PLANS FOR LOCATIONS AND EXTENTS
4. TR = 'TRIMMER' STUDS DIRECTLY BELOW HEADER (BEARING)
5. JAMB STUDS ARE FULL HEIGHT ALONGSIDE THE HEADER
6. ALL JAMBS SHALL BE CONTINUED TO FOUNDATION/LOWEST LEVEL OF FRAMING
7. SEE NOTE 5/S3.1 FOR ADDITIONAL NOTES & DETAILS

8 S3.1 HEADER SCHEDULE
SCALE: NOT APPLICABLE

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HAMMOND, INDIANA

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SHEET DESCRIPTION
FRAMING DETAILS

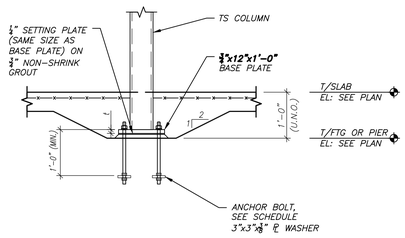
SHEET NUMBER
S3.1
SHEET X of X

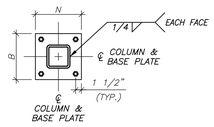
SCALE: 3/4" = 1'-0"

COLUMN SCHEDULE

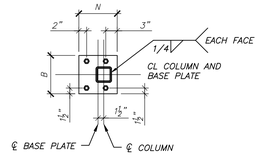
Fy = SEE NOTES

LEVEL \ COLUMN MARK	C1	C2	C2	
ROOF				
SECOND FLOOR	HSS 4x4x¼	HSS 4x4x¼		
FOUNDATION				
BASE PLATE t x N x B	1" x 10" x 10"	1" x 10" x 10"		
ANCHOR BOLTS (4) ¾" DIA. (U.N.O.)	(4) ¾" DIA.	(4) ¾" DIA.		
BASE PLATE DETAIL	A	B		

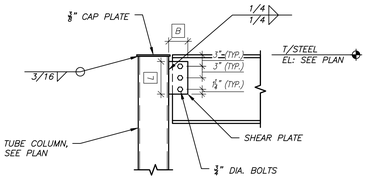




DETAIL A



DETAIL B

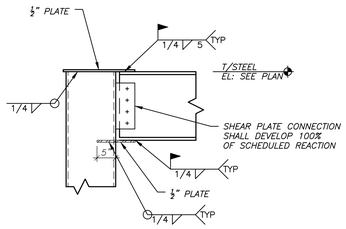


BEAM CONNECTION SCHEDULE

BEAM SIZE	PLATE WIDTH (B)	PLATE LENGTH (L)	PLATE THICKNESS	BOLT SIZE	NUMBER OF BOLTS
W12	4½"	8½"	⅜"	¾"	3
W10	4½"	5½"	⅜"	¾"	2

1 S3.2 COLUMN SCHEDULE

SCALE: NOT APPLICABLE



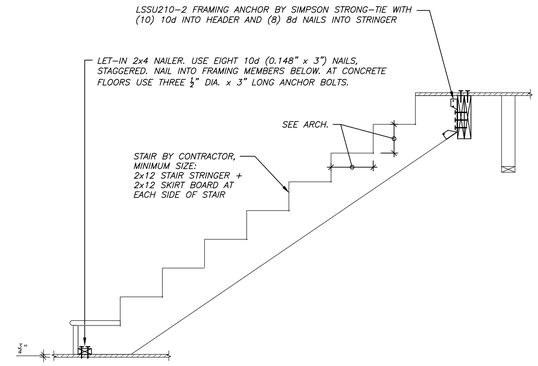
NOTE:
ASD MOMENT: 40 KIP-FT

ALL PLATES SHALL HAVE A MINIMUM THICKNESS EQUAL TO THE BEAM FLANGE

FOR BEAMS LOWER THAN THE TOP OF COLUMN PROVIDE PLATES TOP AND BOTTOM.

2 S3.2 TYPICAL TUBE COLUMN BASE PLATE DETAIL

SCALE: 3/4" = 1'-0"



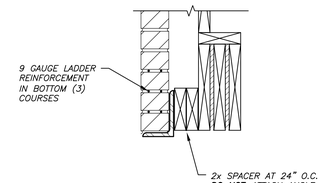
LINTEL SCHEDULE

MARK	WALL TYPE	SPAN	SIZE	DETAIL	REMARKS
L1	BRICK VENEER	< 6 FT	∠6x4x½ (LLV)	A	–
L2	BRICK VENEER	< 12 FT	∠6x4x⅝ (LLV)	A	–

NOTES:
1. SEE ARCHITECTURAL DRAWING FOR WALL OPENING LOCATIONS, LINTEL ELEVATIONS, AND ADDITIONAL LINTEL INFORMATION.
2. INSTALL ANGLE LINTELS LONG LEGS VERTICAL, U.N.O.
3. PROVIDE 4" MINIMUM BEARING AT EACH END OF LINTEL, U.N.O.
4. FULLY GROUT 2 COURSES OF CMU BELOW LINTEL BEARING.
5. PROVIDE LIGHT GAUGE SILL TO SPAN OPENING

3 S3.2 TYPICAL BEAM TO TUBE COLUMN CONNECTION

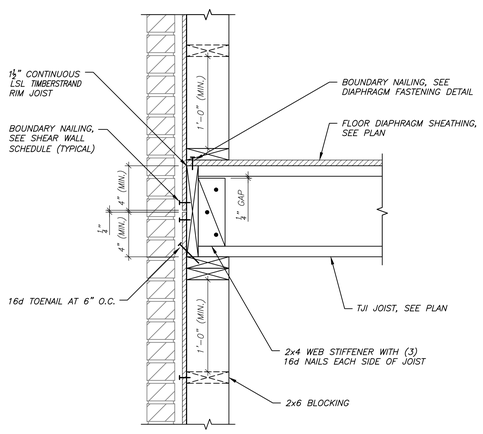
SCALE: NOT TO SCALE



DETAIL A

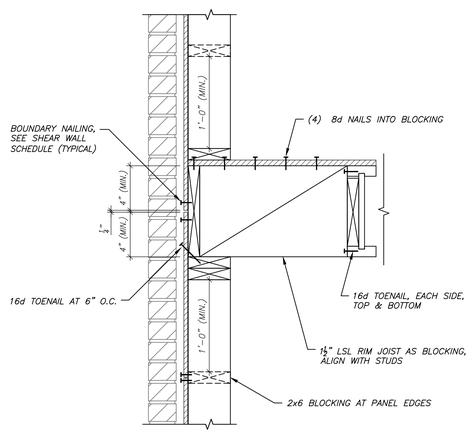
4 S3.2 TYPICAL BEAM TO COLUMN MOMENT CONNECTION

SCALE: NOT TO SCALE



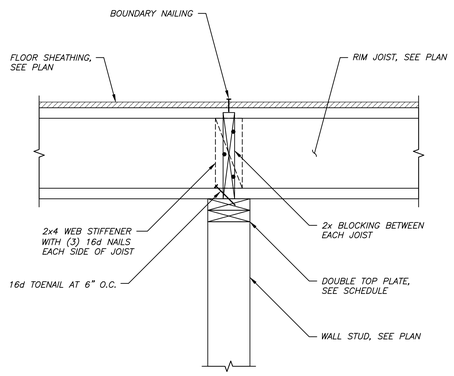
5 S3.2 STAIR STRINGER DETAIL

SCALE: 3/4" = 1'-0"



6 S3.2 LINTEL SCHEDULE

SCALE: NOT APPLICABLE



7 S3.2 SECTION

SCALE: 1 1/2" = 1'-0"

8 S3.2 SECTION

SCALE: 1 1/2" = 1'-0"

9 S3.2 SECTION

SCALE: 1 1/2" = 1'-0"

SCALE: 1-1/2" = 1'-0"

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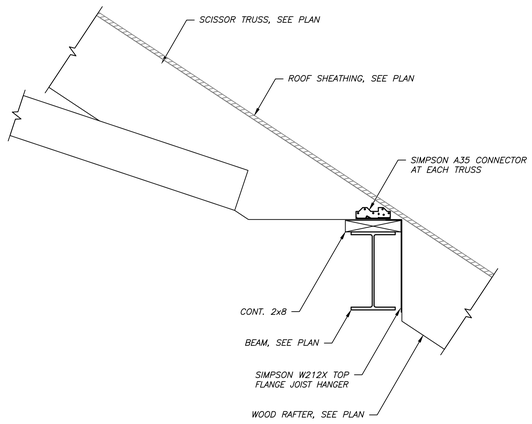
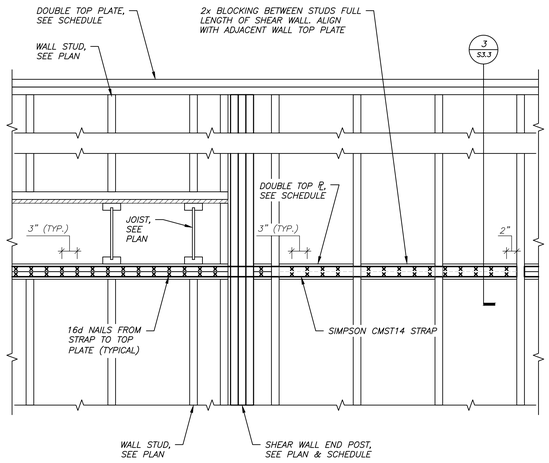
SCALE: 1" = 1'-0"

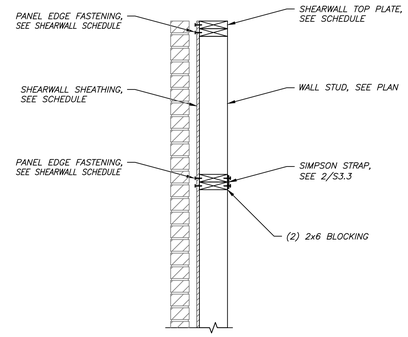


1 / S3.3 SECTION
SCALE: 1 1/2" = 1'-0"

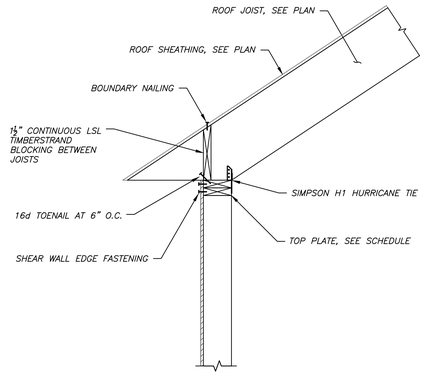


2 / S3.3 ELEVATION
SCALE: 1" = 1'-0"

(something's not right here!)



3 / S3.3 SECTION
SCALE: 1" = 1'-0"



4 / S3.3 SECTION
SCALE: 1" = 1'-0"

5 / S3.3 SECTION
SCALE: 1 1/2" = 1'-0"

NOTE:
SEE 5/S3.3 FOR ADDITIONAL INFORMATION

6 / S3.3 SECTION
SCALE: 1 1/2" = 1'-0"

NOTE:
SEE 5/S3.3 FOR ADDITIONAL INFORMATION

7 / S3.3 SECTION
SCALE: 1 1/2" = 1'-0"

SCALE: 1-1/2" = 1'-0"

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